A Guide to Sizing a Stormwater Quality Manufactured Treatment Device

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Your BMP Toolbox

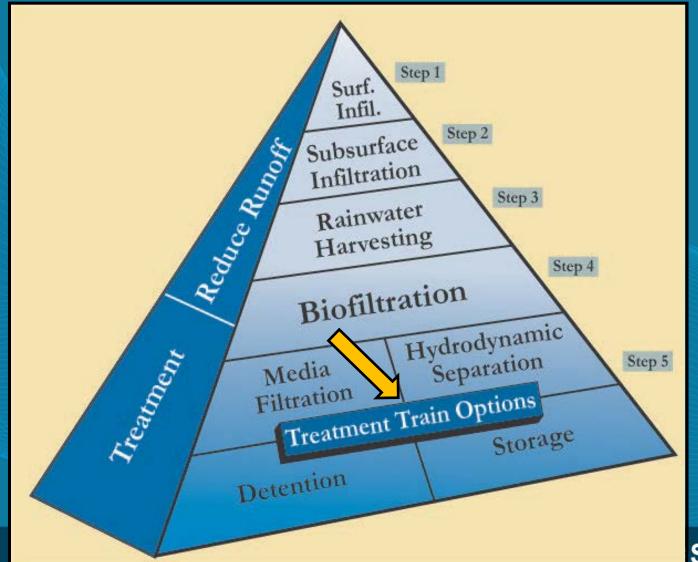
One of the leading topics today in stormwater management is a drive for sustainability, green infrastructure and Low Impact Development (LID) practices.

Let's reach into our "BMP toolbox" to consider Manufactured Treatment Devices (MTDs) in LID designs and how to properly size them for long term water quality performance.





Low Impact Development (LID) Technology Selection Pyramid





LID Benefits & Technologies

	LID Benefits						
Technology	Capture Rainfall	Reduce Peak Flows	Reduce Runoff Volumes	Enhance Infiltration	Filter Out Pollutants		
Surface Infiltration	X	X	X	X	X		
Underground Infiltration	X	X	X	X	X		
Rainwater Harvesting & Storage	X	X	X				
Biofiltration	X	X	X	X	X		
Media Filtration					X		

Hydrodynamic Separation can be used for Pretreatment for Underground Infiltration and Detention



HDS Pretreatment



Harvesting



Above ground storage



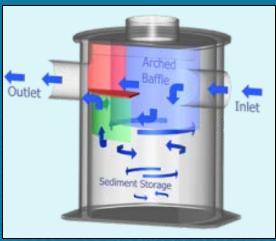


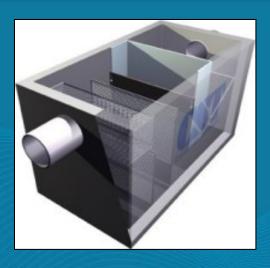
Underground storage and infiltration

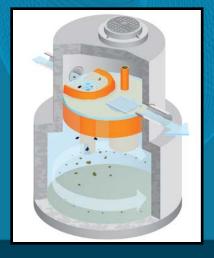


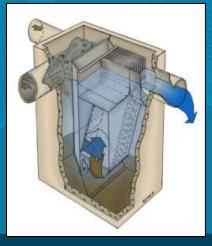
Hydrodynamic Separators (HDSs)















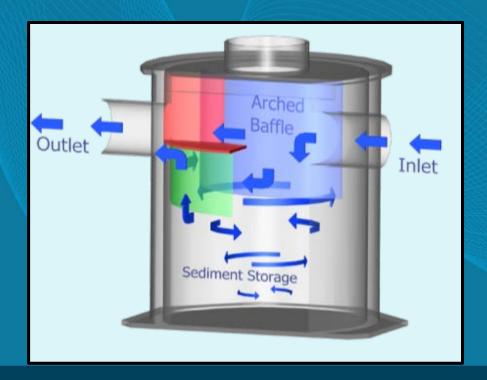


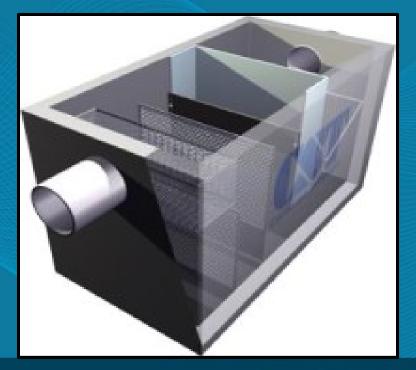
Types of HDSs

Captures sediment, debris, floatables, oil

Vortex type =
Gravitational &
Centrifugal Forces

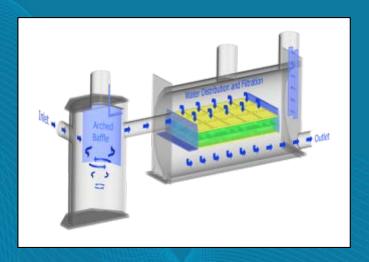
<u>Vault type</u> = Gravitational Forces

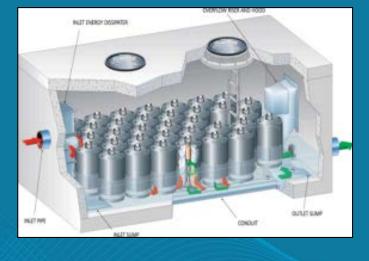


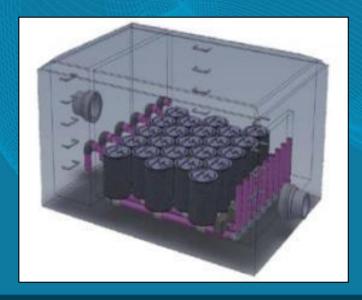




Media Filtration



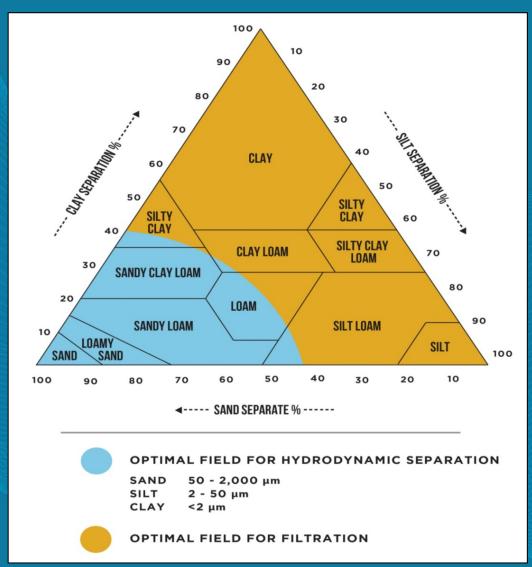








HDSs & Filters vs. Soil Types





HDSs ≥ 50 μm, Challenged <40-50 μm

Filters $< 75 \mu m$, challenged $< 15-20 \mu m$



You need to know...

1. Water Quality Flow Rate (Q in cfs) that the MTD is to treat.

For example, using the Rational Method:

Q = CIA, where:

C = Runoff coefficient (ex.: 0.9 impervious)

I = Intensity (in/hr, regulated or derived)

A = Inflow drainage area, acres



- 2. Peak flow rate that exceeds the water quality flow rate (Q).
- 3. Conveyance piping network and pipe diameters for Q and peak flows.
- 4. Conveyance piping elevations & slopes. MTD must fit into facility conditions.

These elements lead to ...

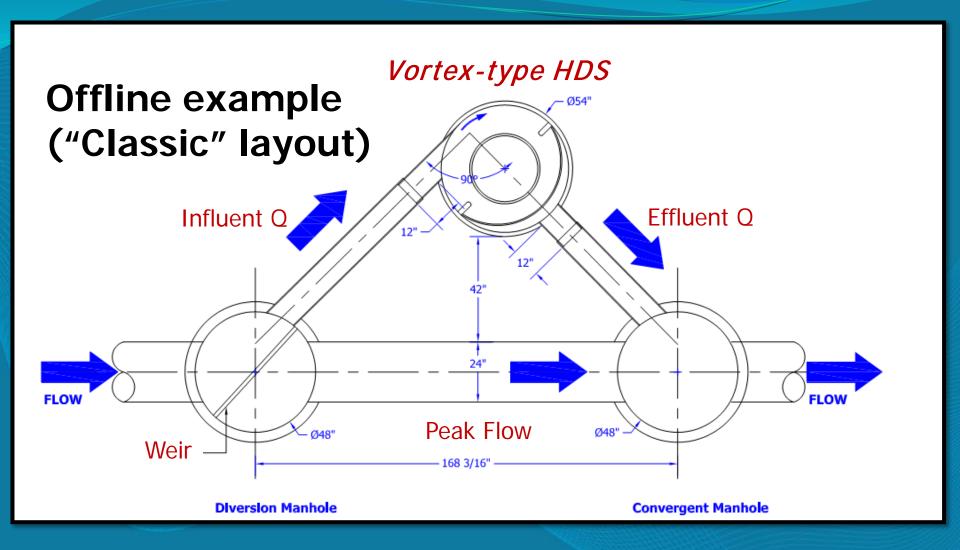


5. <u>MTD layout in either offline or online (inline)</u> <u>configuration.</u>

<u>Offline MTDs</u>

- ✓ Some MTD internal components may differ between offline or online applications. Check with manufacturer.
- ✓ Offline MTDs convey up to Q only, flow >Q bypasses MTD.
- **✓** Offline MTDs use diversion and convergent flow structures.
- ✓ Offline MTD pipe diameters < peak flow pipe diameters.
- ✓ Some regulations only allow offline MTDs since online units may be, or perceived to be, more susceptible to scouring (resuspension) of previously captured sediment.
- **✓** Offline layouts usually > footprint than online layouts.
- ✓ Offline layouts not always >>\$\$ than online layouts.







5. MTD layout in either offline or online (inline) configuration.

Online MTDs

- ✓ Online MTD treats Q and internally conveys untreated bypass flows >Q (no external piping).
- ✓ Online MTD footprint usually < offline MTD footprint, favorable for retrofits if online allowed.
- ✓ Online MTDs pipe diameters > offline Q pipe diameters since online designs must convey all flows.
- ✓ Some regulations prohibit online MTD designs.
- ✓ Scour (re-suspension) testing data usually required to allow for online MTD installation (NJDEP).
- ✓ Online layouts not always << \$\$ than offline layouts.



Consequences of PSD Specification

<u>Undersizing</u>

- ➤ Potential for diminished performance and increased potential for scouring, especially for online.
- Concern for runoff conveyance (tailwater backup, upstream flooding) due to potentially undersized piping associated with the MTD (offline or online).
- Leads to increased maintenance frequency due to decreased storage capacity and long term functionality which increase operational costs.



Consequences of PSD Specification

Oversizing

- ➤ Increases footprint, can be a problem for tight spaces and/or retrofits.
- Increases project costs from a properly sized device.
- Conservative TSS removal efficiency.
- ➤ Pollutant loading will ultimately dictate maintenance frequency and cost.

Trash Only

If PSD specification is too coarse, maximum hydraulic capacity may be exceeded causing catastrophic failure.





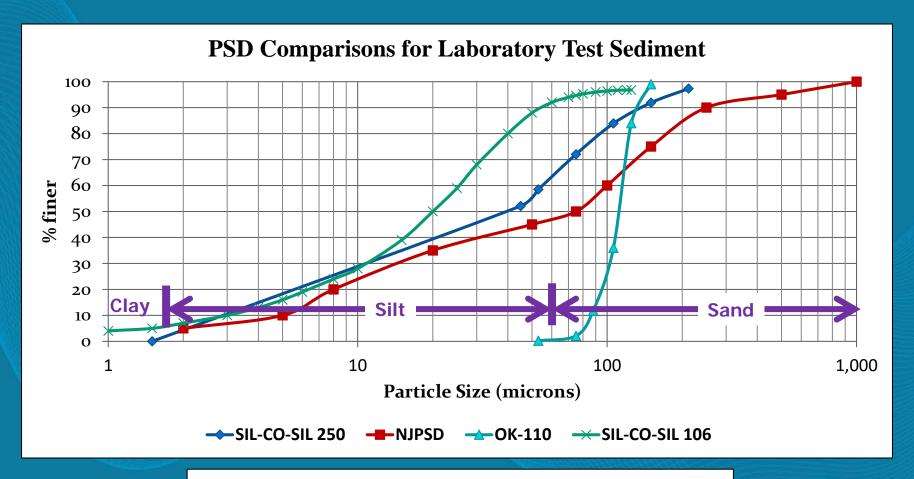
80% removal of what?

Many stormwater manuals require 80% TSS removal using non-structural and/or structural BMPs. However, manuals often fail to specify the <u>particle</u> <u>size distribution (PSD)</u> that is to be removed. Let's take a look at how different PSDs impact performance curves and <u>sizing</u> of HDSs.





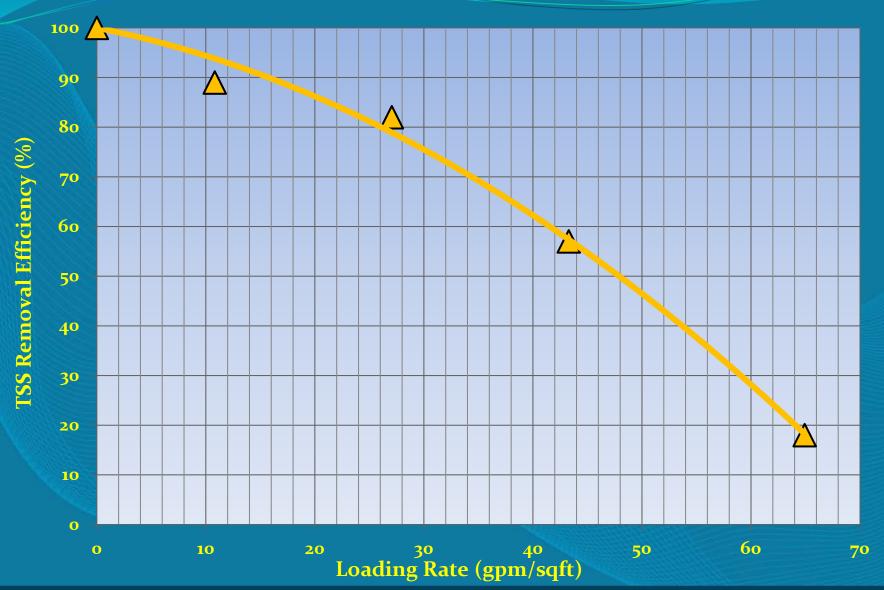
What is Particle Size Distribution?



So, which PSD is your basis for MTD sizing?



HDS Performance Curve: OK-110 Test Sediment





Where I got the idea to use Peclet Number

University of Minnesota ST. ANTHONY FALLS LABORATORY

Engineering, Environmental and Geophysical Fluid Dynamics

PROJECT REPORT No. 494

Performance Assessment of Underground Stormwater Treatment Devices

By

Matthew A. Wilson, John S. Gulliver, Omid Mohseni, and Ray M. Hozalski



Prepared for Local Road Research Board and Twin Cities Metropolitan Council

> July 2007 Minneapolis, Minnesota



What's the Peclet Number?

- ❖ Provides a simple means to predict HDS performance using a different particle size than that of the test sediment.
- Allows for HDS sizing charts for different PSDs.
- Performance curves from different HDSs having different test sediment PSDs can be compared.



Peclet Number (Pe)

$$Pe = (d \cdot h \cdot Vs) / Q$$

d = Horizontal flow dimension in feet

h = Vertical flow dimension in feet

Vs = Particle settling velocity in feet/sec

Q = Flow rate in cubic feet/second

- "d" in Vortex HDS = diameter of effective treatment area
- "d" in Vault HDS = long axis of effective treatment area (parallel to flow)



Calculate Pe for Tested HDS

Test Parameters	Q (cfs)	Loading Rate (gpm/ft²) TSS RE (%)		Pe (unitless)	
$d_{50} = 110 \ \mu m \ (OK-110)$	0	0	100	NA	
Vs = 0.021 ft/s	0.20	10.8	89	1.33	
SG = 2.65	0.50	27.1	82	0.53	
$\mathbf{d} = 3.3 \text{ ft}$	0.80	43.3	57	0.33	
h = 3.83 ft	1.20	64.9	18	0.22	

 $Pe = (d \cdot h \cdot Vs) / Q$

Example: Q = 0.2 cfs

 $Pe = (3.3 \text{ ft} \cdot 3.83 \text{ ft} \cdot 0.021 \text{ ft/sec}) / 0.2 \text{ cfs} = 1.33$



Calculate Particle Settling Velocity (Vs)

Term	Variable	Units	Description
Gs	2.65		Specific gravity of particle
ρ_{s}	165.07	lb/ft³	Density of particle
ρ_{w}	62.29	lb/ft³	Density of water
g	32.20	ft/s²	Acceleration due to gravity
T	20.00	C°	Temperature of water
T	68	F°	Temperature of water
μ	2.09E-05	lb*s/ft ²	Dynamic viscosity of water at given temp.
U	1.08E-05	ft ² /s	Kinematic Viscosity of water
D	110	micron	Diameter of particle
Vs	0.024	ft/s	Settling velocity, Cheng Formula
Vs	0.02080	ft/s	Settling velocity, Stoke's Law
Vs	0.029	ft/s	Settling velocity, Ferguson & Church



Stoke's Law Particle Settling Velocities

Faster settling time

Particle Size	Vs
(µm)	(ft/sec)
45	0.0085
50	0.010
67	0.013
75	0.014
90	0.017
110	0.021
125	0.024



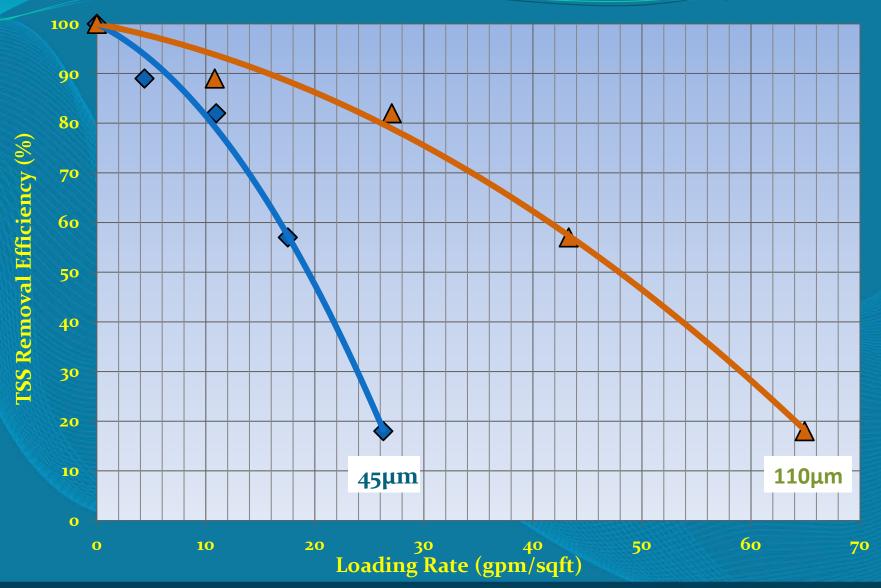
Performance Summary - 45 µm

Rearrange equation to solve for Q Q = (3.3 ft - 3.83 ft - Vs) / Pe RE and Pe constant

Parameters	Q (cfs)	Loading Rate (gpm/ft²)	RE (%)	Pe (unitless)
$d_{50} = 45 \mu m$	0	0	100	NA
Vs = 0.0085 ft/sec	0.081	4.4	89	1.33
SG = 2.65	0.202	10.9	82	0.53
$d = 3.3 \text{ ft } (8.3 \text{ ft}^2)$	0.325	17.5	57	0.33
h = 3.83 ft	0.486	26.3	18	0.22



HDS Performance Curves for 45 and 110 µm





50 μm							
Donomotoro	Q	Loading Rate	RE	Pe			
Parameters	(cfs)	(gpm/ft²)	(%)	(unitless)			
$d_{50} = 50 \ \mu m$	0	0	100	NA			
Vs = 0.010 ft/sec	0.10	5.2	89	1.33			
SG = 2.65	0.24	12.9	82	0.53			
$\mathbf{d} = 3.3 \; \mathbf{ft}$	0.38	20.6	57	0.33			
h = 3.83 ft	0.57	30.9	18	0.22			

67 μm (Old d ₅₀ from NJDEP PSD)						
	Q	Loading Rate	RE	Pe		
Parameters	(cfs) (gpm/ft²)		(%)	(unitless)		
$d_{50} = 67 \mu m$	0	0	100	NA		
Vs = 0.0.013 ft/sec	0.124	6.7	89	1.33		
SG = 2.65	0.310	16.7	82	0.53		
d = 3.3 ft	0.495	26.8	57	0.33		
h = 3.83 ft	0.743	40.2	18	0.22		



75 μm							
Danamatana	Q Loading Rate		RE	Pe			
Parameters	(cfs)	(gpm/ft²)	(%)	(unitless)			
$d_{50} = 75 \mu m$	0	0	100	NA			
Vs = 0.014 ft/sec	0.133	7.2	89	1.33			
SG = 2.65	0.333	18.0	82	0.53			
$\mathbf{d} = 3.3 \text{ ft}$	0.533	28.9	57	0.33			
h = 3.83 ft	0.800	43.3	18	0.22			

90 μm						
Donomotoro	Q	Loading Rate	RE	Pe		
Parameters	(cfs)	(gpm/ft²)	(%)	(unitless)		
$d_{50} = 90 \mu m$	0	0	100	NA		
Vs = 0.017 ft/sec	0.162	8.8	89	1.33		
SG = 2.65	0.405	21.9	82	0.53		
d = 3.3 ft	0.648	35.0	57	0.33		
h = 3.83 ft	0.971	52.6	18	0.22		

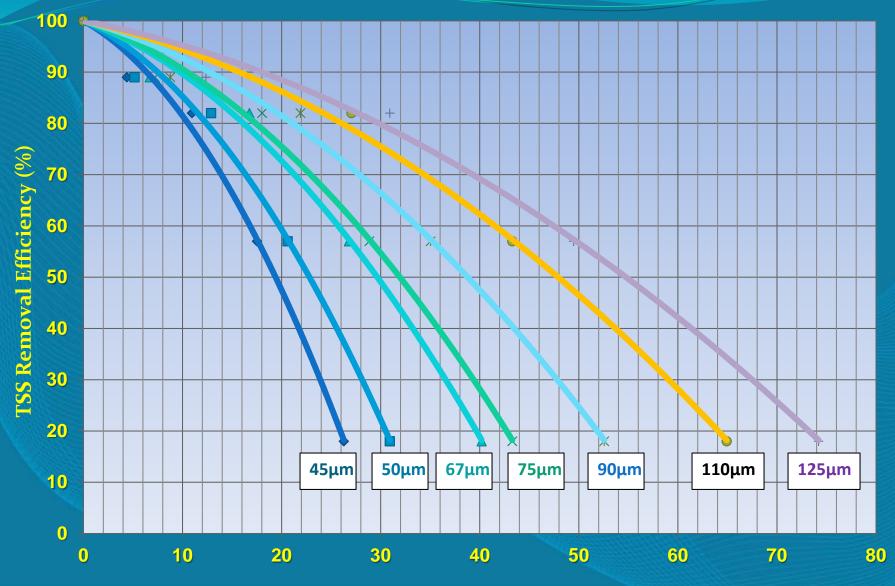


110 μm							
Daramatara	Q	Loading Rate	RE	Pe			
Parameters	(cfs)	(gpm/ft²)	(%)	(unitless)			
$d_{50} = 110 \ \mu m$	0	0	100	NA			
Vs = 0.021 ft/sec	0.2	10.8	89	1.33			
SG = 2.65	0.5	27.1	82	0.53			
$\mathbf{d} = 3.3 \text{ ft}$	0.8	43.3	57	0.33			
h = 3.83 ft	1.2	64.9	18	0.22			

125 μm							
Parameters	Q	Loading Rate	RE	Pe			
	(cfs)	(gpm/ft²)	(%)	(unitless)			
$d_{50} = 125 \ \mu m$	0	0	100	NA			
Vs = 0.024 ft/sec	0.229	12.4	89	1.33			
SG = 2.65	0.571	30.9	82	0.53			
$\mathbf{d} = 3.3 \text{ ft}$	0.914	49.5	57	0.33			
h = 3.83 ft	1.371	74.2	18	0.22			



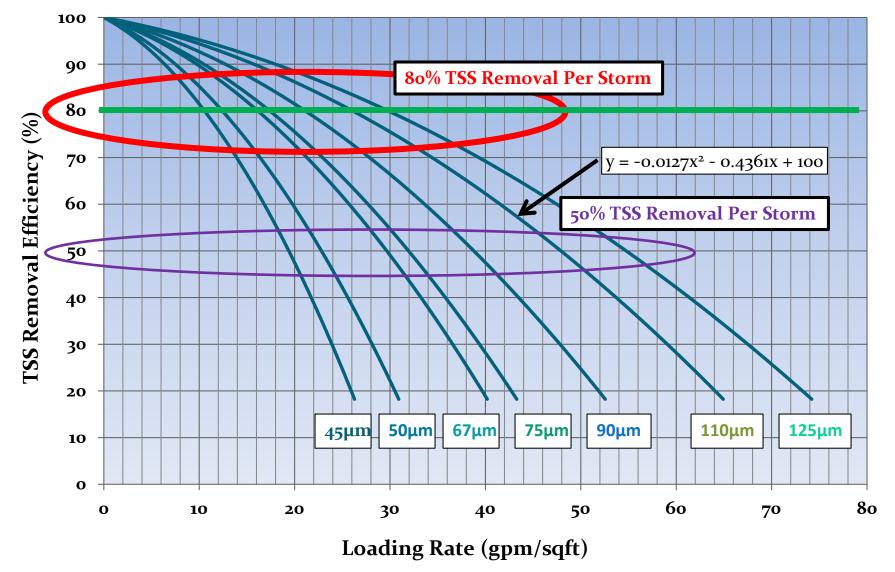
HDS Performance Curves for Different Particle Sizes



Loading Rate (gpm/sqft)



HDS 80% TSS Removal Per Storm





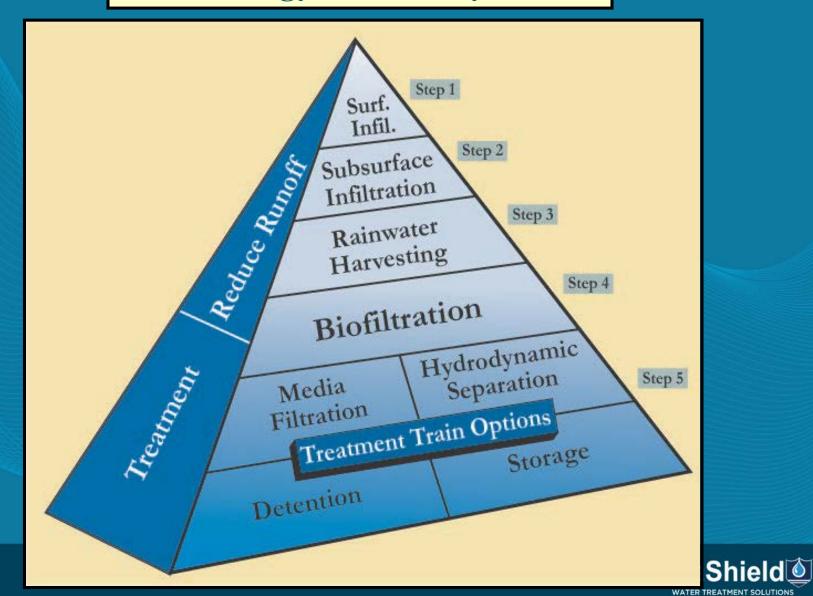
HDS Sizing Charts: 80% TSS Removal per Storm

				Particle Si	ze and Lo	ading Rate	e	
Example HDS	Effective	45 μm	50 μm	67 µm	75 μm	90 µm	110 µm	125 µm
Model Diameter (ft)	Treatment Area (ft²)	10.5 gpm/ft ²	12.2 gpm/ft ²	16.0 gpm/ft ²	17.5 gpm/ft ²	21.0 gpm/ft ²	26.0 gpm/ft ²	30.0 gpm/ft ²
(11)		Q (cfs)	Q (cfs)	Q (cfs)				
4.0	12.6	0.29	0.34	0.45	0.49	0.59	0.73	0.84
5.0	19.6	0.46	0.53	0.70	0.76	0.92	1.14	1.31
6.0	28.3	0.66	0.77	1.01	1.10	1.32	1.64	1.89
8.0	50.3	1.18	1.37	1.79	1.96	2.35	2.91	3.36
10.0	78.5	1.84	2.13	2.80	3.06	3.67	4.54	5.24

Q (cfs) = (square $ft \cdot gpm/sq ft$) / 448.83 gpm/cfs



Low Impact Development (LID) Technology Selection Pyramid



It's all about good clean water...



Tennessee River, Chattanooga



Thank you.



INNOVATING GOOD CLEAN WATER

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